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DRAINAGE COMPUTATIONS SHOE CARNIVAL DISTRIBUTION CENTER

NE Corner of SR 57 and Ruston Lane
Vanderburgh County, Indiana

Prepared by:

American Consulting, Inc.
7260 Shadeland Station
Indianapolis, IN 46256-3397

February 27, 2006
Revised: October 4, 2006

Submitted By: Tony Halsey, PE

RECEIVED BY THE
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SURVEYOR'S OFFICE

10/6/06 am Bf

**DRAINAGE COMPUTATIONS
SHOE CARNIVAL DISTRIBUTION CENTER
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DRAINAGE COMPUTATIONS SHOE CARNIVAL DISTRIBUTION CENTER

Project Description

Woodward Commercial Realty proposes to construct a 400,000-sft warehouse at the northeast corner of State Road 57 and Ruston Lane near Daylight, in Vanderburgh County, Indiana. The site is bounded on the west by SR 57, on the south by Ruston Lane, and on the east by a railroad right-of-way and I-164 right-of-way. The warehouse will also contain more than 8,000-sft of office space and will employ a maximum of 200 people. Accordingly, parking facilities will be provided for 200 employees. The main entrance for the facility will be off of Ruston Lane. Ruston Lane will be widened to three lanes from SR 57 approximately 150' east to the site entrance as part of this project.

A portion of this property lies in a 100-year floodplain, as delineated on FIRM Map Number 180256 0015 C, revised August 5, 1991. However, the floodplain is north of the limits of this project. No portion of the current project lies within the FEMA delineated 100-year flood area. The northern edge of the truck parking lot lies below elevation 395.0 feet, which is the flood elevation established by the Vanderburgh County Building Commissioner.

Existing Conditions

The site is an abandoned coal processing facility. Existing infrastructure on the site includes an asphalt driveway, some concrete pads, an old shed, some chain link fence, and three small ponds. The ponds range in size from 0.15 to 1.4 acres. A larger pond, roughly six acres in size, is located at the north end of the site. Existing transformers, buried electric/telephone lines, and overhead electrical/telephone lines are located throughout the southern half of the site. A large underground petroleum line crosses east and west through the site just south of the largest pond.

Existing groundcover is a mixture of native grasses, trees, and agriculture. Generally, the southern portion of the site is covered with grass, the middle section with a mixture of trees and grass, and the northern portion with a mixture of grass and agriculture. About 850' north of Ruston Lane, a 48-inch box culvert daylights from under SR 57 into an open swale. The swale enters a 72" CMP and flows east across the site into a large swale along the railroad right-of-way.

There are four main drainage areas on site. Approximately 2.5 acres drain via overland flow south to Ruston Lane, 9.8 acres drain via overland flow north and east into the six acre pond and/or the pond's outfall in the I-164 right-of-way, and 12.8 acres drain through the existing swale/72" CMP into large swale along the railroad right-of-way. The remainder of the site is contained in depressional storage in one of the three smaller ponds. If the ponds overflow, the overflow would go into the I-164 right-of-way.

For the purposes of meeting Vanderburgh County's ordinance, the 12.8-acre watershed draining to the railroad and the 9.8-acre watersheds draining north were used for existing conditions. The final drainage outlets after construction will be to these two points. ICPR was used to calculate an existing 10-year discharge to both points. ICPR is a computer stormwater modeling program that uses hydrograph routing methods, similar to TR-20 or HEC-HMS. Huff third-quartile (50 percent) distributions were used for hydrograph routing. Results show the existing peak 10-year discharge to the railroad right-of-way is 9.02 cfs, and the existing peak 10-year discharge to the north is 6.98 cfs. These numbers will be used as the allowable post-construction peak 25-year discharges.

Table 1—Existing 10-Year Peak Stormwater Discharges

Basin/Discharge Point	Peak Q (cfs)
<i>Railroad R/W</i>	9.02 cfs
<i>6-Acre Pond</i>	6.98 cfs

There are also several identified wetlands on this property. Mitigation of these wetlands, as well as relocation of the open swale across the property, has been coordinated through the Army Corps of Engineers. Wetlands south of the existing fence north of the 1.4-acre pond have been mitigated, while all wetlands north of the fence are not mitigated. These wetlands will remain after completion of the project.

Proposed Conditions

The proposed site will be divided into two basic drainage basins. The first basin will be approximately 27.3 acres and ultimately drain to the same open swale to which the existing 12.8-acre watershed drains. Offsite drainage from under SR 57 will be piped through the site and will also outlet to this swale, as in existing conditions.

South Basin

All areas south of the proposed building, including the access drive off of Ruston Lane and the employee parking lot, will sheet drain into a dry detention basin south of the parking lot. After being detained, stormwater will reconnect with offsite bypass flow and be piped around the southeast corner of the building into the existing swale.

The proposed building and the truck dock area will drain via a combination of ditch flow and sheet flow into a detention pond northeast of the building. The proposed detention pond will be constructed out of the 1.4 acre existing pond (the biggest of the three small ponds). The southwest corner of the existing pond will be filled in, and a linear "finger" will be constructed off the northwest corner to create the new pond. The outlet structure for the detention pond will be a 24" RCP with a 12" orifice pipe grouted into place.

The emergency overflow will be on the north bank of the pond, which is where the existing pond overflows. If the emergency overflow is used, the overflow will drain into I-164 right-of-way, then south into the swale along the railroad.

North Basin

The northernmost 10.5 acres of the site, including all truck parking areas north of the proposed building, will drain north into the existing 6-acre pond and/or its outfall in the I-164 right-of-way. Runoff from the truck parking lot will be detained in a small detention pond off the northeast corner of the parking lot prior to discharging into the larger 6-acre pond. The outfall from the detention pond will be a 12" RCP culvert.

This pond will be constructed adjacent to a delineated FEMA floodplain and will discharge into that floodplain. Although FEMA has not published a 100-year flood elevation, the Vanderburgh County Building Commissioner established a 100-year flood elevation of 395.0 feet. However, no 25-year flood elevation was established. To determine the 25-year flood elevation, an ICPR model was developed analyzing offsite flow through the 6-acre pond and its outfall. Results from that model estimate the 25-year flood elevation is 391.65 feet, which is 3.35 feet lower than the 100-year flood elevation set by the Building Commissioner. Calculations for the 25-year flood can be found in the appendix.

To ensure the pond functions effectively during the 25-year storm, a 10-wide berm with a top elevation of 392.5 will be built around the pond. The northern bank will be six inches lower than the banks around the rest of the pond. The entire northern bank of the pond will serve as the emergency spillway. Overflow capacity calculations can be found in the appendix of this report.

Results

ICPR was used to analyze inflow/outflow rates and maximum stages in each detention basin, as well as combined runoff from each basin to the respective discharge points. Results show during a 25-year storm event, the peak runoff rate is maintained well below the existing 10-year runoff rate for each basin.

The tables below provide a summary of the detention ponds and total runoff.

Table 2 – Individual Detention Basin Summary

Basin	Outlet Point	Initial Stage (ft)	Max. Stage (ft)	Max. 25-year Q (cfs)	Time to Max. Stage/Q (hrs)
Dry Pond	Railroad R/W	389.63 (dry)	392.82	3.69	1.87
Pond	Railroad R/W	389.5 (wet)	392.45	4.91	6.05
Far North Pond	6-Acre Pond	389.0 (wet)	390.99	5.55	1.89

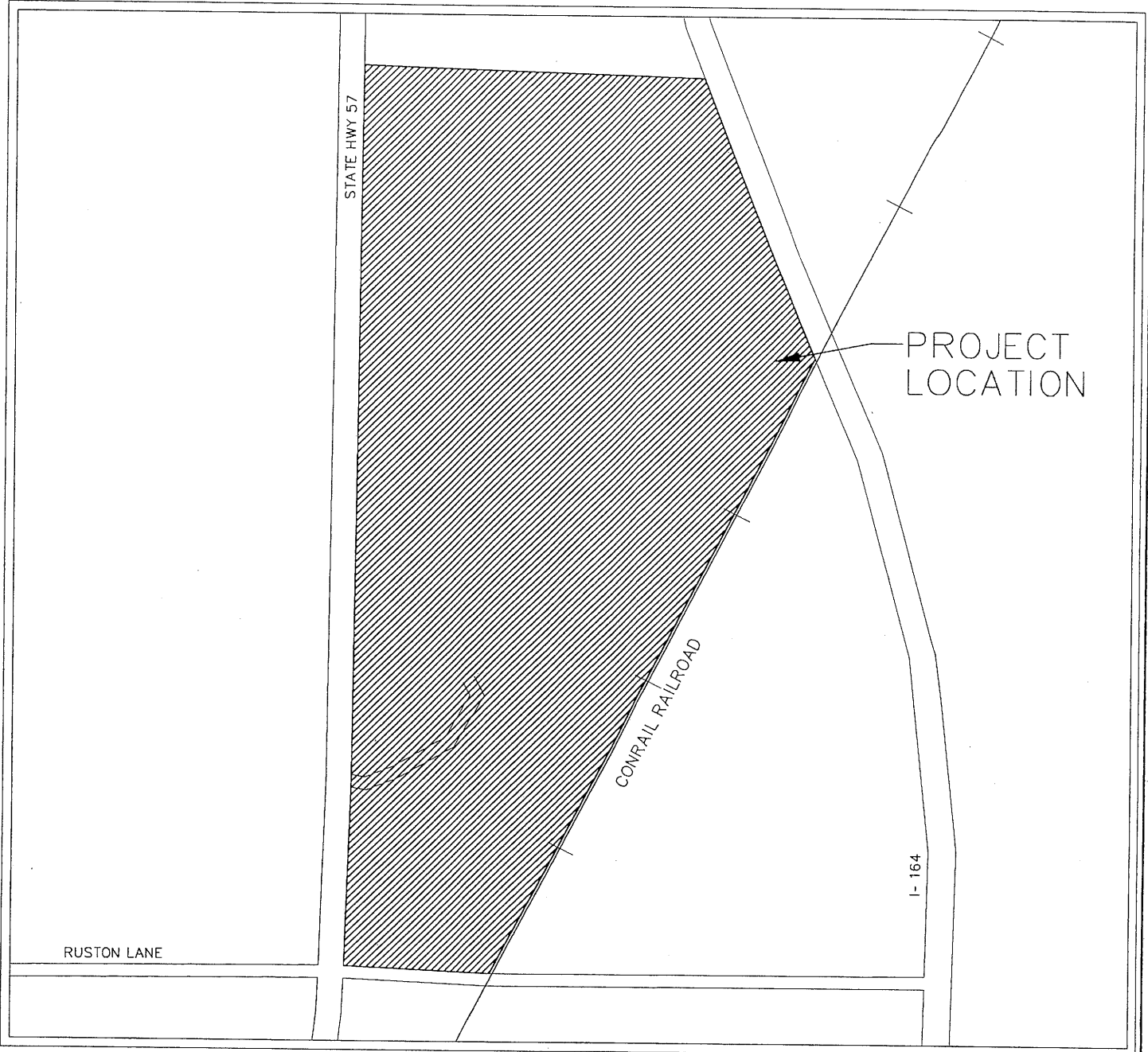
Table 3 – Allowable/Actual Stormwater Discharge

Outlet Point	Allowable 25-year Q (cfs)	Proposed 25-year Q (cfs)
Railroad R/W	9.02	8.16
6-Acre Pond	6.98	5.55

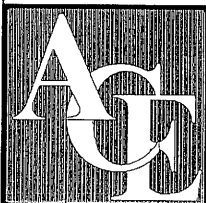
Conclusion

No adverse impacts are anticipated from this project.

**APPENDIX A:
VICINITY MAP**



VICINITY MAP



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SCALE: N.T.S

DATE: 02/27/2006

DRAWN BY: TH

CHK'D. BY: TH

JOB NO. IN2005.0589

SHEET NO.

OF

**APPENDIX B:
FLOOD INSURANCE RATE MAP**

CEMETERY

U.S. Coast and Geodetic stamped "112" set in 74 feet east across No. corner of the Ivor Jagu telephone pole: southw. centerline of North Gree edge of a concrete head rail of the conrail tracks approximately 1 foot below 6 inches below the level

397.342

ELEVATION (FEET NGVD)

ELEVATION REFERENCE MARKS

DESC

NATIONAL FLOOD INSURANCE PROGRAM

FIRM FLOOD INSURANCE RATE MAP

VANDEBURGH COUNTY, INDIANA UNINCORPORATED AREAS

PANEL 15 OF 100

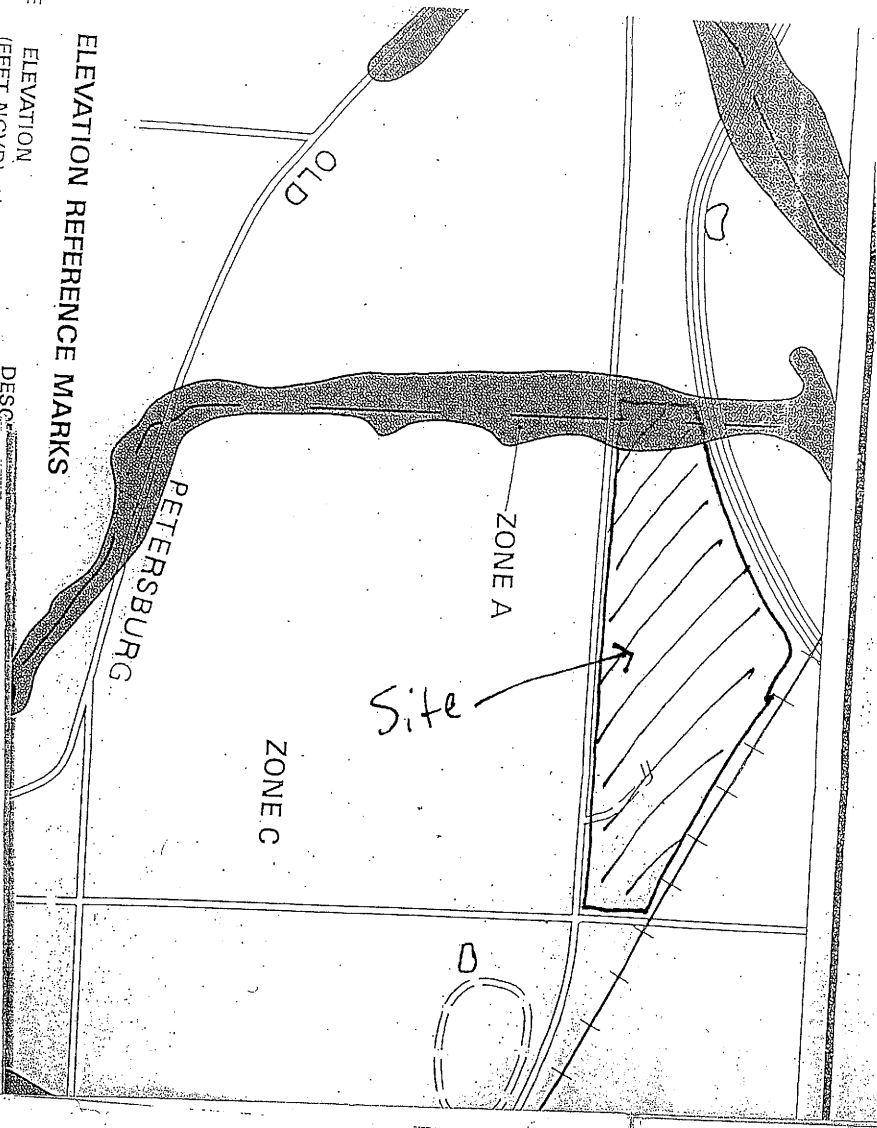
COMMUNITY-PANEL NUMBER

180256 0015 C

MAP REVISED:

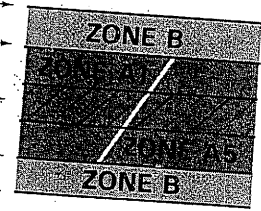
AUGUST 5, 1991

Federal Emergency Management Agency



KEY TO MAP

- 500-Year Flood Boundary
- 100-Year Flood Boundary
- Floodway
- 100-Year Flood Boundary
- 500-Year Flood Boundary
- Base Flood Elevation Line With Elevation In Feet**
- Base Flood Elevation in Feet Where Uniform Within Zone**
- Elevation Reference Mark
- Zone D Boundary
- River Mile



513

(EL 987)

RM7x

M1.5

**Referenced to the National Geodetic Vertical Datum of 1929

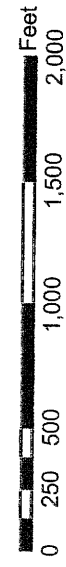
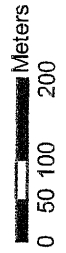
*EXPLANATION OF ZONE DESIGNATIONS

ZONE	EXPLANATION
A	Areas of 100-year flood; base flood elevations and flood hazard factors not determined.
A0	Areas of 100-year shallow flooding where depths are between one (1) and three (3) feet; average depth of inundation are...

**APPENDIX C:
SOILS TYPE MAP**

SOIL SURVEY OF VANDERBURGH COUNTY, INDIANA

Shoe Carnival Offsite Area



SOIL SURVEY OF VANDERBURGH COUNTY, INDIANA

Shoe Carnival Offsite Area

MAP LEGEND

- Soil Map Units
- Cities
- Detailed Counties
- Detailed States
- Interstate Highways
- Roads
- Rails
- Water
- Hydrography
- Oceans
- Escarpment, bedrock
- Escarpment, non-bedrock
- Gully
- Levee
- Slope
- Blowout
- Borrow Pit
- Clay Spot
- Depression, closed
- Eroded Spot
- Gravel Pit
- Gravelly Spot
- Gully
- Lava Flow
- Landfill
- Marsh or Swamp
- Miscellaneous Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Slide or Slip
- Sinkhole
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Perennial Water
- Wet Spot

MAP INFORMATION

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL: <http://websoilsurvey.nrcs.usda.gov>

Coordinate System: UTM Zone 16

Soil Survey Area: Vanderburgh County, Indiana
 Spatial Version of Data: 1
 Soil Map Compilation Scale: 1:15840

Map comprised of aerial images photographed on these dates:
 1998

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend Summary

Vanderburgh County, Indiana

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Ba	Bartle silt loam	14.3	5.7
Bd	Birds silt loam	9.3	3.7
Bo	Bonnie silt loam	1.2	0.5
He	Henshaw silt loam	5.0	2.0
HoA	Hosmer silt loam, 0 to 2 percent slopes	20.8	8.2
HoB2	Hosmer silt loam, 2 to 6 percent slopes, eroded	110.0	43.7
HoB3	Hosmer silt loam, 2 to 6 percent slopes, severely eroded	40.1	15.9
HoC3	Hosmer silt loam, 6 to 12 percent slopes, severely eroded	7.7	3.0
St	Stendal silt loam	31.8	12.6
ZaC3	Zanesville silt loam, 6 to 12 percent slopes, severely eroded	11.8	4.7

Runoff Curve Number (CN)

Project: Shoe Carnival Warehouse
 Location: Offsite

By: TH
 Checked: _____

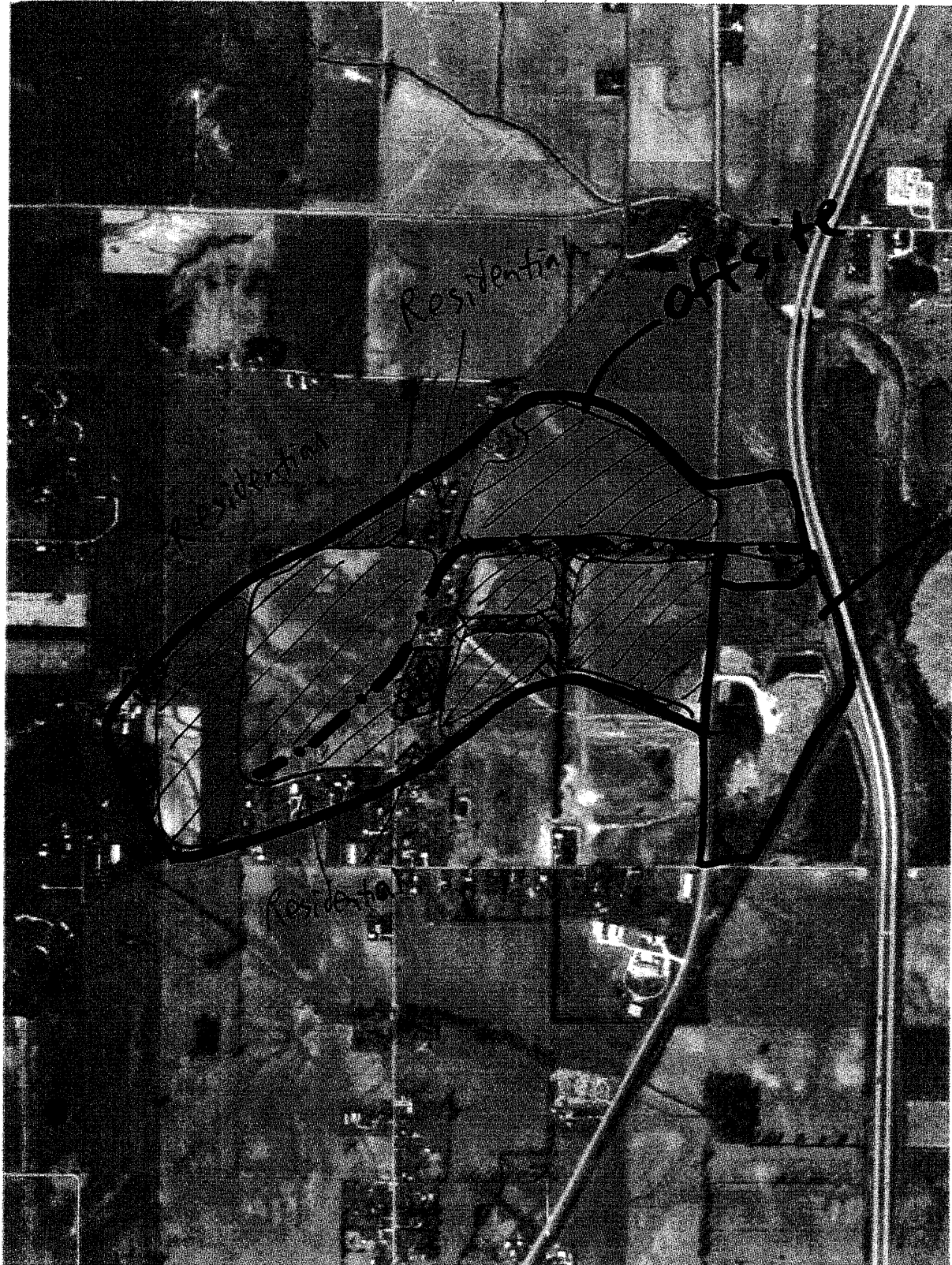
Date: 8/10/2006
 Date: _____

Check one: _____

Soil Symbol	Soil Name	Hydrologic Group	Area	
			____ acres ____ sq.mi. __x__ %	Acreage
Ba	Bartle Silt Loam	D	5.7	14.4
Bd	Birds Silt Loam	C	3.7	9.3
Bo	Bonnie Silt Loam	C	0.5	1.3
He	Henshaw Silt Loam	C	2.0	5.0
HoA	Hosmer Silt Loam, 0-2% slopes	C	8.2	20.7
HoB2	Hosmer Silt Loam, 2-6% slopes, eroded	C	43.7	110.1
HoB3	Hosmer Silt Loam, 2-6% slopes, severely eroded	C	15.9	40.1
HoC3	Hosmer Silt Loam, 6-12% slopes, severely eroded	C	3.0	7.6
St	Stendal Silt Loam	C	12.6	31.8
ZaC3	Zanesville Silt Loam, 5-12% slopes, severely eroded	C	4.7	11.8
			100.00	252

Soil Group	% of Site
A	0
B	0
C	94.3
D	5.7

Send To Printer Back To TerraServer Change to 11x17 Print Size Show Grid Lines Change to Landscape
USGS 17 km NE of Evansville, Indiana, United States 24 Feb 1998



0 0.5 Km

0 0.25 Mi

Runoff Curve Number (CN)

Project: Shoe Carnival Warehouse
 Location: Offsite

By: TH
 Checked: _____

Date: 8/10/2006
 Date: _____

Check one: Present Developed _____

Soil Name and Hydrologic Group (Appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN			Area	Product of CN x area
		Table 2-2	Figure 2-3	Figure 2-4	____ acres ____ sq.mi. x ____ %	
C	Residential, 1/2 acres lots	80			10	800
C	Row crops & crop residue	85			74.3	6315.5
D	Row crops & crop residue	89			5.7	507.3
C	Woods	72			10	720
Totals =					100.00	8342.8

CN (weighted) = $\frac{\text{total product}}{\text{total area}}$ = 83.43

Use CN = 83

*Notes

Time of Concentration (T_c) or Travel Time (T_t)

Project: Shoe Carnival Warehouse By: TH Date: 1/26/2006
 Location: Off site Checked: _____ Date: _____

Check one: Present Developed _____
 Check one: T_c T_t _____ through subarea

Sheet Flow

1. Surface description (Table 3-1)
2. Manning's roughness coeff., n (Table 3-1)
3. Flow Length, L (L < 300 ft)
4. Two-year 24-hr rainfall, P₂
5. Land slope, s
6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$

Segment ID				
Range				
	0.13			
ft	300			
in	2.64			
ft/ft	0.017			
hr	0.42	+		= 0.42

Shallow Concentrated Flow

7. Surface description, (paved or unpaved)
8. Flow length, L
9. Watercourse slope, s
10. Average velocity, V (Figure 3-1)
11. $T_t = \frac{L}{3600 V}$

Segment ID			
	unpaved	unpaved	
ft	800	700	
ft/ft	0.038	0.014	
ft/s	3.1	1.9	
hr	0.07	0.10	+ = 0.17

Channel Flow

12. Cross sectional flow area, a
13. Wetted perimeter, P_w
14. Hydraulic radius, r = a/P_w
15. Channel slope, s
16. Mannings roughness coeff., n
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$
18. Flow length, L
19. $T_t = \frac{L}{3600 V}$

Segment ID			
ft ²			
ft ²			
ft ²			
ft/ft			
ft/s	3		
ft	4000		
hr	0.37	+	+ = 0.37

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, and 19)

hr	0.96
min	57.6

**APPENDIX D:
EXISTING CONDITIONS MAP**

**APPENDIX E:
EXISTING CN AND TC CALCULATIONS**

Time of Concentration (T_c) or Travel Time (T_t)

Project: Shoe Carnival Warehouse By: TH Date: 2/23/2006
 Location: On site Checked: _____ Date: _____

Check one: Present Developed _____
 Check one: T_c T_t _____ through subarea

Sheet Flow

1. Surface description (Table 3-1)
2. Manning's roughness coeff., n (Table 3-1)
3. Flow Length, L (L < 300 ft)
4. Two-year 24-hr rainfall, P₂
5. Land slope, s
6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$

Segment ID					
Range					
	0.13	0.13			
ft	255	45			
in	2.64	2.64			
ft/ft	0.014	0.067			
hr	0.39	+ 0.05			= 0.45

Shallow Concentrated Flow

7. Surface description, (paved or unpaved)
8. Flow length, L
9. Watercourse slope, s
10. Average velocity, V (Figure 3-1)
11. $T_t = \frac{L}{3600 V}$

Segment ID					
unpaved					
ft	460				
ft/ft	0.015				
ft/s	2				
hr	0.06	+		+	= 0.06

Channel Flow

12. Cross sectional flow area, a
13. Wetted perimeter, P_w
14. Hydraulic radius, r = a/P_w
15. Channel slope, s
16. Mannings roughness coeff., n
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$
18. Flow length, L
19. $T_t = \frac{L}{3600 V}$

Segment ID					
ft ²	12				
ft ²	12.65				
ft ²	0.95				
ft/ft	0.004				
	0.03				
ft/s	2.95				
ft	525				
hr	0.05	+		+	= 0.05

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, and 19) hr 0.56
min 33.6

Time of Concentration (T_c) or Travel Time (T_t)

Project: Shoe Carnival Warehouse By: TH Date: 4/24/2006
 Location: Far North Checked: _____ Date: _____

Check one: Present Developed _____
 Check one: T_c T_t _____ through subarea

Sheet Flow

1. Surface description (Table 3-1)
2. Manning's roughness coeff., n (Table 3-1)
3. Flow Length, L (L < 300 ft)
4. Two-year 24-hr rainfall, P₂
5. Land slope, s
6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$

Segment ID					
	Range				
	0.13				
	ft	300			
	in	2.64			
	ft/ft	0.025			
	hr	0.35	+		= 0.35

Shallow Concentrated Flow

7. Surface description, (paved or unpaved)
8. Flow length, L
9. Watercourse slope, s
10. Average velocity, V (Figure 3-1)
11. $T_t = \frac{L}{3600 V}$

Segment ID					
	unpaved				
	ft	435			
	ft/ft	0.006			
	ft/s	1.2			
	hr	0.10	+		= 0.10

Channel Flow

12. Cross sectional flow area, a
13. Wetted perimeter, P_w
14. Hydraulic radius, r = a/P_w
15. Channel slope, s
16. Mannings roughness coeff., n
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$
18. Flow length, L
19. $T_t = \frac{L}{3600 V}$

Segment ID					
	ft ²				
	ft ²				
	ft ²				
	ft/ft				
	ft/s				
	ft				
	hr		+		= 0.00

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, and 19) hr 0.45
min 27.2

**APPENDIX F:
PROPOSED CONDITIONS MAP**

**APPENDIX G:
PROPOSED CN AND TC CALCULATIONS**

Runoff Curve Number (CN)

Project: Shoe Carnival Warehouse
 Location: Onsite South

By: TH
 Checked: _____

Date: 2/23/2006
 Date: _____

Check one: Present Developed

Soil Name and Hydrologic Group (Appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN			Area	Product of CN x area
		Table 2-2	Figure 2-3	Figure 2-4	___ x ___ acres ___ sq.mi. ___ %	
C	Yard	74			2.98	220.52
	Impervious	98			2.38	233.24
Totals =					5.36	453.76

CN (weighted) = $\frac{\text{total product}}{\text{total area}}$ = 84.66 Use CN = **85**

*Notes

Runoff Curve Number (CN)

Project: Shoe Carnival Warehouse
 Location: Onsite North

By: TH
 Checked: _____

Date: 2/23/2006
 Date: _____

Check one: Present Developed

Soil Name and Hydrologic Group (Appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN			Area	Product of CN x area
		Table 2-2	Figure 2-3	Figure 2-4	_____ x _____ acres _____ sq.mi. _____ %	
C	Yard	74			7.01	518.74
	Impervious	98			14.01	1372.98
Totals =					21.02	1891.72

CN (weighted) = $\frac{\text{total product}}{\text{total area}}$ = 90.00

Use CN = 90

*Notes

Runoff Curve Number (CN)

Project: Shoe Carnival Warehouse
 Location: Onsite Far North

By: TH
 Checked: _____

Date: 9/6/2006
 Date: _____

Check one: Present Developed

Soil Name and Hydrologic Group (Appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN			Area	Product of CN x area
		Table 2-2	Figure 2-3	Figure 2-4	___ x ___ acres ___ sq.mi. ___ %	
C	Yard	74			7.28	538.942
	Impervious	98			3.18	311.248
Totals =					10.46	850.19

CN (weighted) = $\frac{\text{total product}}{\text{total area}}$ = 81.29

Use CN = 81

*Notes

Time of Concentration (T_c) or Travel Time (T_t)

Project: Shoe Carnival Warehouse By: TH Date: 2/23/2006
 Location: South Checked: _____ Date: _____

Check one: Present Developed
 Check one: T_c T_t through subarea

Sheet Flow

1. Surface description (Table 3-1)
2. Manning's roughness coeff., n (Table 3-1)
3. Flow Length, L (L < 300 ft)
4. Two-year 24-hr rainfall, P₂
5. Land slope, s
6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$

Segment ID	Road				
	Range				
	0.013				
	ft	270			
	in	2.88			
	ft/ft	0.012			
	hr	0.07	+		= 0.07

Shallow Concentrated Flow

7. Surface description, (paved or unpaved)
8. Flow length, L
9. Watercourse slope, s
10. Average velocity, V (Figure 3-1)
11. $T_t = \frac{L}{3600 V}$

Segment ID					
	ft				
	ft/ft				
	ft/s				
	hr				= 0.00

Channel and Pipe Flow

12. Cross sectional flow area, a
13. Wetted perimeter, P_w
14. Hydraulic radius, r = a/P_w
15. Channel slope, s
16. Mannings roughness coeff., n
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$
18. Flow length, L
19. $T_t = \frac{L}{3600 V}$

Segment ID	Ditch 1	Pipe			
	ft ²	0.7854			
	ft ²	16.12			
	ft ²	0.496			
	ft/ft	0.010			
		0.03			
	ft/s	3.10			
	ft	265			
	hr	0.02	+	0.01	= 0.03

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, and 19)

hr	0.10
min	6.1

Time of Concentration (T_c) or Travel Time (T_t)

Project: Shoe Carnival Warehouse By: TH Date: 2/23/2006
 Location: North Checked: _____ Date: _____

Check one: Present Developed
 Check one: T_c T_t through subarea

Sheet Flow

1. Surface description (Table 3-1)
2. Manning's roughness coeff., n (Table 3-1)
3. Flow Length, L (L < 300 ft)
4. Two-year 24-hr rainfall, P₂
5. Land slope, s
6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$

Segment ID					
	Range				
	0.13				
	ft	150			
	in	2.88			
	ft/ft	0.080			
	hr	0.12	+		= 0.12

Shallow Concentrated Flow

7. Surface description, (paved or unpaved)
8. Flow length, L
9. Watercourse slope, s
10. Average velocity, V (Figure 3-1)
11. $T_t = \frac{L}{3600 V}$

Segment ID					
	unpaved	unpaved	paved		
	ft	150	335	720	
	ft/ft	0.0033	0.013	0.0025	
	ft/s	0.93	1.87	1.02	
	hr	0.04	+	0.05	+
				0.20	= 0.29

Channel Flow

12. Cross sectional flow area, a
13. Wetted perimeter, P_w
14. Hydraulic radius, r = a/P_w
15. Channel slope, s
16. Mannings roughness coeff., n
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$
18. Flow length, L
19. $T_t = \frac{L}{3600 V}$

Segment ID					
	ft ²	12			
	ft ²	12.65			
	ft ²	0.95			
	ft/ft	0.005			
		0.03			
	ft/s	3.38			
	ft	750			
	hr	0.06	+		+
					= 0.06

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, and 19) hr 0.47
min 28.5

**APPENDIX H:
STORM SEWER CAPACITY CALCULATIONS**

**APPENDIX I:
ICPR INPUT AND RESULTS**

Name	Group	Simulation	Max Time Stage hrs	Max Stage ft	Warning Stage ft	Max Delta Stage ft	Max Surf Area ft2	Max Time Inflow hrs	Max Inflow cfs	Max Time Outflow hrs	Max Outflow cfs
Pond N	BASE	25-24hr	16.80	389.908	391.500	0.0026	14191	16.74	3.34	16.80	3.20
Pond N	BASE	25-2hr	1.89	390.994	391.500	0.0038	16978	1.42	18.54	1.89	5.55
Pond N	BASE	25-3hr	2.70	390.989	391.500	0.0039	16963	2.08	14.40	2.70	5.54
Pond N	BASE	25-6hr	4.80	390.781	391.500	0.0048	16421	4.17	9.04	4.80	5.16
Proposed	BASE	10-12hr	0.00	380.000	381.000	0.0000	7	8.74	6.10	0.00	0.00
Proposed	BASE	10-1hr	0.00	380.000	381.000	0.0000	7	1.29	5.24	0.00	0.00
Proposed	BASE	10-24hr	0.00	380.000	381.000	0.0000	7	16.83	5.47	0.00	0.00
Proposed	BASE	10-2hr	0.00	380.000	381.000	0.0000	7	2.15	6.95	0.00	0.00
Proposed	BASE	10-3hr	0.00	380.000	381.000	0.0000	7	3.02	7.12	0.00	0.00
Proposed	BASE	10-6hr	0.00	380.000	381.000	0.0000	7	5.08	6.88	0.00	0.00
Proposed	BASE	25-12hr	0.00	380.000	381.000	0.0000	7	9.18	6.95	0.00	0.00
Proposed	BASE	25-1hr	0.00	380.000	381.000	0.0000	7	1.31	6.10	0.00	0.00
Proposed	BASE	25-24hr	0.00	380.000	381.000	0.0000	7	16.85	6.08	0.00	0.00
Proposed	BASE	25-2hr	0.00	380.000	381.000	0.0000	7	2.17	7.84	0.00	0.00
Proposed	BASE	25-3hr	0.00	380.000	381.000	0.0000	7	3.00	8.16	0.00	0.00
Proposed	BASE	25-6hr	0.00	380.000	381.000	0.0000	7	5.00	7.86	0.00	0.00
Proposed N	BASE	10-12hr	0.00	380.000	381.000	0.0000	1	8.37	3.74	0.00	0.00
Proposed N	BASE	10-1hr	0.00	380.000	381.000	0.0000	1	1.04	3.61	0.00	0.00
Proposed N	BASE	10-24hr	0.00	380.000	381.000	0.0000	1	16.55	3.03	0.00	0.00
Proposed N	BASE	10-2hr	0.00	380.000	381.000	0.0000	1	1.89	4.79	0.00	0.00
Proposed N	BASE	10-3hr	0.00	380.000	381.000	0.0000	1	2.69	4.81	0.00	0.00
Proposed N	BASE	10-6hr	0.00	380.000	381.000	0.0000	1	4.34	4.50	0.00	0.00
Proposed N	BASE	25-12hr	0.00	380.000	381.000	0.0000	1	8.38	4.27	0.00	0.00
Proposed N	BASE	25-1hr	0.00	380.000	381.000	0.0000	1	1.04	4.38	0.00	0.00
Proposed N	BASE	25-24hr	0.00	380.000	381.000	0.0000	1	16.80	3.20	0.00	0.00
Proposed N	BASE	25-2hr	0.00	380.000	381.000	0.0000	1	1.89	5.55	0.00	0.00
Proposed N	BASE	25-3hr	0.00	380.000	381.000	0.0000	1	2.70	5.54	0.00	0.00
Proposed N	BASE	25-6hr	0.00	380.000	381.000	0.0000	1	4.80	5.16	0.00	0.00

Peak RR
 25yr runoff into pond to north
 Peak 25yr runoff into 6-acre pond to north

Name	Group	Simulation	Max Time Stage hrs	Max Stage ft	Warning Stage ft	Max Delta Stage ft	Max Surf Area ft2	Max Time Inflow hrs	Max Inflow cfs	Max Time Outflow hrs	Max Outflow cfs
Dry Pond	BASE	10-12hr	8.35	391.072	394.000	0.0050	4955	8.25	2.59	8.35	2.25
Dry Pond	BASE	10-1hr	1.02	391.882	394.000	0.0050	6364	0.73	10.21	1.02	2.97
Dry Pond	BASE	10-24hr	16.77	390.421	394.000	0.0050	2388	16.74	1.61	16.77	1.60
Dry Pond	BASE	10-2hr	1.85	392.329	394.000	0.0050	7141	1.42	9.01	1.85	3.33
Dry Pond	BASE	10-3hr	2.50	392.200	394.000	0.0050	6916	2.08	6.99	2.50	3.23
Dry Pond	BASE	10-6hr	4.32	391.772	394.000	0.0050	6173	4.17	4.37	4.32	2.88
Dry Pond	BASE	25-12hr	8.37	391.378	394.000	0.0050	5487	8.25	2.99	8.37	2.53
Dry Pond	BASE	25-1hr	1.03	392.307	394.000	0.0050	7103	0.73	12.82	1.03	3.32
Dry Pond	BASE	25-24hr	16.78	390.530	394.000	-0.0050	2840	16.74	1.84	16.78	1.82
Dry Pond	BASE	25-3hr	1.87	392.817	394.000	0.0050	7989	1.42	10.89	1.87	3.69
Dry Pond	BASE	25-6hr	2.54	392.662	394.000	-0.0050	7720	2.08	8.35	2.54	3.58
Dry Pond	BASE	25-6hr	4.33	392.166	394.000	-0.0050	6857	4.17	5.13	4.33	3.20
Existing	BASE	10-12hr	0.00	380.000	381.000	0.0000	0	8.50	4.45	0.00	0.00
Existing	BASE	10-1hr	0.00	380.000	381.000	0.0000	0	1.10	6.22	0.00	0.00
Existing	BASE	10-24hr	0.00	380.000	381.000	0.0000	0	16.83	2.99	0.00	0.00
Existing	BASE	10-2hr	0.00	380.000	381.000	0.0000	0	1.75	2.02	0.00	0.00
Existing	BASE	10-3hr	0.00	380.000	381.000	0.0000	0	2.33	8.38	0.00	0.00
Existing	BASE	10-6hr	0.00	380.000	381.000	0.0000	0	4.33	6.64	0.00	0.00
Existing	BASE	25-12hr	0.00	380.000	381.000	0.0000	0	8.50	5.35	0.00	0.00
Existing	BASE	25-1hr	0.00	380.000	381.000	0.0000	0	1.07	8.75	0.00	0.00
Existing	BASE	25-24hr	0.00	380.000	381.000	0.0000	0	16.83	3.53	0.00	0.00
Existing	BASE	25-2hr	0.00	380.000	381.000	0.0000	0	1.75	11.79	0.00	0.00
Existing	BASE	25-3hr	0.00	380.000	381.000	0.0000	0	2.33	10.78	0.00	0.00
Existing	BASE	25-6hr	0.00	380.000	381.000	0.0000	0	4.33	8.24	0.00	0.00
Existing N	BASE	10-12hr	0.00	380.000	381.000	0.0000	0	8.50	3.32	0.00	0.00
Existing N	BASE	10-1hr	0.00	380.000	381.000	0.0000	0	1.03	4.99	0.00	0.00
Existing N	BASE	10-24hr	0.00	380.000	381.000	0.0000	0	16.83	2.24	0.00	0.00
Existing N	BASE	10-2hr	0.00	380.000	381.000	0.0000	0	1.67	6.98	0.00	0.00
Existing N	BASE	10-3hr	0.00	380.000	381.000	0.0000	0	2.25	6.43	0.00	0.00
Existing N	BASE	10-6hr	0.00	380.000	381.000	0.0000	0	4.33	4.98	0.00	0.00
Existing N	BASE	25-12hr	0.00	380.000	381.000	0.0000	0	8.50	3.99	0.00	0.00
Existing N	BASE	25-1hr	0.00	380.000	381.000	0.0000	0	1.03	7.06	0.00	0.00
Existing N	BASE	25-24hr	0.00	380.000	381.000	0.0000	0	16.83	2.64	0.00	0.00
Existing N	BASE	25-2hr	0.00	380.000	381.000	0.0000	0	1.67	9.21	0.00	0.00
Existing N	BASE	25-3hr	0.00	380.000	381.000	0.0000	0	2.25	8.33	0.00	0.00
Existing N	BASE	25-6hr	0.00	380.000	381.000	0.0000	0	4.25	6.21	0.00	0.00
Pond	BASE	10-12hr	10.87	391.952	395.000	0.0033	56335	8.25	11.64	10.87	4.29
Pond	BASE	10-1hr	1.61	390.917	395.000	0.0030	50023	0.97	34.45	1.61	2.76
Pond	BASE	10-24hr	19.26	391.719	395.000	0.0032	54913	16.75	7.12	19.24	3.98
Pond	BASE	10-2hr	2.46	391.657	395.000	-0.0031	54536	1.58	35.59	2.46	3.90
Pond	BASE	10-3hr	3.36	391.863	395.000	0.0036	55795	2.25	29.63	3.36	4.18
Pond	BASE	10-6hr	6.05	392.056	395.000	0.0037	57042	4.25	19.64	6.05	4.42
Pond	BASE	25-12hr	10.88	392.295	395.000	0.0038	58815	8.25	13.27	10.88	4.85
Pond	BASE	25-1hr	1.61	391.217	395.000	0.0030	51855	0.97	42.09	1.61	3.25
Pond	BASE	25-24hr	19.29	392.031	395.000	0.0034	56861	16.74	8.04	19.31	4.39
Pond	BASE	25-2hr	2.46	392.046	395.000	0.0033	56966	1.58	42.36	2.46	4.41
Pond	BASE	25-3hr	3.36	392.257	395.000	0.0032	58534	2.25	34.81	3.36	4.81
Pond	BASE	25-6hr	6.05	392.448	395.000	0.0043	59945	4.25	22.73	6.05	4.99
Pond N	BASE	10-12hr	8.37	390.148	391.500	0.0035	14768	8.25	4.55	8.37	3.74
Pond N	BASE	10-1hr	1.04	390.099	391.500	0.0036	14639	0.73	15.81	1.04	3.61
Pond N	BASE	10-24hr	16.55	389.853	391.500	0.0025	14074	16.74	2.89	16.55	3.03
Pond N	BASE	10-2hr	1.89	390.599	391.500	0.0040	15946	1.42	15.00	1.89	4.79
Pond N	BASE	10-3hr	2.69	390.607	391.500	0.0039	15966	2.08	11.82	2.69	4.81
Pond N	BASE	10-6hr	4.34	390.465	391.500	0.0045	15594	4.17	7.57	4.34	4.50
Pond N	BASE	25-12hr	8.38	390.364	391.500	0.0036	15331	8.25	5.32	8.38	4.27
Pond N	BASE	25-1hr	1.04	390.412	391.500	0.0036	15456	0.73	20.46	1.04	4.38

Existing 10-yr runoff into RR R/W

Existing 10-yr runoff into 6-acre pond to north

394.000 0.2600

Name: Existing Base Flow(cfs): 0.000 Init Stage(ft): 380.000
Group: BASE Warn Stage(ft): 381.000
Type: Time/Stage

Time (hrs)	Stage (ft)
0.00	380.000
99999.00	380.000

Name: Existing N Base Flow(cfs): 0.000 Init Stage(ft): 380.000
Group: BASE Warn Stage(ft): 381.000
Type: Time/Stage

Time (hrs)	Stage (ft)
0.00	380.000
99999.00	380.000

Name: Pond Base Flow(cfs): 0.000 Init Stage(ft): 389.500
Group: BASE Warn Stage(ft): 395.000
Type: Stage/Area

Stage (ft)	Area (ac)
389.500	0.9700
390.000	1.0200
391.000	1.1600
392.000	1.3000
393.000	1.4700

Name: Pond N Base Flow(cfs): 0.000 Init Stage(ft): 389.000
Group: BASE Warn Stage(ft): 391.500
Type: Stage/Area

Stage (ft)	Area (ac)
388.500	0.2600
389.000	0.2800
390.000	0.3300
391.000	0.3900
392.000	0.5500

Name: Proposed Base Flow(cfs): 0.000 Init Stage(ft): 380.000
Group: BASE Warn Stage(ft): 381.000
Type: Time/Stage

Time (hrs)	Stage (ft)
0.00	380.000
99999.00	380.000

Name: Proposed N Base Flow(cfs): 0.000 Init Stage(ft): 380.000
Group: BASE Warn Stage(ft): 381.000
Type: Time/Stage

Time (hrs)	Stage (ft)
0.00	380.000
99999.00	380.000

==== Cross Sections =====

Name: Encroachment: No Group: BASE

Span(in): 12.00 Invert(ft): 389.500
Rise(in): 12.00 Control Elev(ft): 389.500

Name: Oultet N From Node: Pond N Length(ft): 50.00
Group: BASE To Node: Proposed N Count: 1

UPSTREAM	DOWNSTREAM	Friction Equation: Average Conveyance
Geometry: Circular	Circular	Solution Algorithm: Automatic
Span(in): 18.00	18.00	Flow: None
Rise(in): 18.00	18.00	Entrance Loss Coef: 0.000
Invert(ft): 388.500	388.000	Exit Loss Coef: 0.000
Manning's N: 0.013000	0.013000	Outlet Ctrl Spec: Use dc or tw
Top Clip(in): 0.000	0.000	Inlet Ctrl Spec: Use dn
Bot Clip(in): 0.000	0.000	Solution Incs: 10

Upstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall

Downstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall

*** Weir 1 of 1 for Drop Structure Oultet N ***

Count: 1	Bottom Clip(in): 0.000	TABLE
Type: Vertical: Mavis	Top Clip(in): 0.000	
Flow: Both	Weir Disc Coef: 3.200	
Geometry: Circular	Orifice Disc Coef: 0.600	
Span(in): 12.00	Invert(ft): 388.500	
Rise(in): 12.00	Control Elev(ft): 388.500	

==== Weirs =====

Name: From Node:
Group: BASE To Node:
Flow: Both Count: 1
Type: Horizontal Geometry: Circular

Span(in): 0.00
Rise(in): 0.00
Invert(ft): 0.000
Control Elevation(ft): 0.000

TABLE

Bottom Clip(in): 0.000
Top Clip(in): 0.000
Weir Discharge Coef: 3.200
Orifice Discharge Coef: 0.600

==== Hydrology Simulations =====

Name: 10-12hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-12hr.R32

Override Defaults: Yes
Storm Duration(hrs): 12.00
Rainfall File: Huff 3
Rainfall Amount(in): 3.55

Time(hrs)	Print Inc(min)
3.000	15.00
8.000	5.00
15.000	15.00

Name: 10-1hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-1hr.R32

Override Defaults: Yes
Storm Duration(hrs): 1.00
Rainfall File: Huff 3
Rainfall Amount(in): 1.82

Time(hrs)	Print Inc(min)
-----------	----------------

1.500 2.00
2.000 5.00

Name: 10-24hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-24hr.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Huff 3
Rainfall Amount(in): 4.18

Time(hrs)	Print	Inc(min)
8.000		15.00
18.000		5.00
30.000		15.00

Name: 10-2hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-2hr.R32

Override Defaults: Yes
Storm Duration(hrs): 2.00
Rainfall File: Huff 3
Rainfall Amount(in): 2.46

Time(hrs)	Print	Inc(min)
2.000		5.00
4.000		15.00

Name: 10-3hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-3hr.R32

Override Defaults: Yes
Storm Duration(hrs): 3.00
Rainfall File: Huff 3
Rainfall Amount(in): 2.70

Time(hrs)	Print	Inc(min)
1.000		15.00
3.000		5.00
5.000		15.00

Name: 10-6hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-6hr.R32

Override Defaults: Yes
Storm Duration(hrs): 6.00
Rainfall File: Huff 3
Rainfall Amount(in): 3.13

Time(hrs)	Print	Inc(min)
2.000		15.00
6.000		5.00
9.000		15.00

Name: 100-12hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\100-12hr.R32

Override Defaults: Yes
Storm Duration(hrs): 12.00
Rainfall File: Huff 3
Rainfall Amount(in): 4.92

Time(hrs)	Print	Inc(min)
3.000		15.00
8.000		5.00
15.000		15.00

Name: 100-1hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\100-1hr.R32

Override Defaults: Yes
Storm Duration(hrs): 1.00
Rainfall File: Huff 3
Rainfall Amount(in): 2.41

Time(hrs)	Print Inc(min)
1.500	2.00
2.000	5.00

 Name: 100-24hr
 Filename: D:\IN2005\0985-Shoe Carnival\ICPR\100-24hr.R32

Override Defaults: Yes
 Storm Duration(hrs): 24.00
 Rainfall File: Huff 3
 Rainfall Amount(in): 5.52

Time(hrs)	Print Inc(min)
8.000	15.00
18.000	5.00
30.000	15.00

 Name: 100-2hr
 Filename: D:\IN2005\0985-Shoe Carnival\ICPR\100-2hr.R32

Override Defaults: Yes
 Storm Duration(hrs): 2.00
 Rainfall File: Huff 3
 Rainfall Amount(in): 3.57

Time(hrs)	Print Inc(min)
2.000	5.00
4.000	15.00

 Name: 100-3hr
 Filename: D:\IN2005\0985-Shoe Carnival\ICPR\100-3hr.R32

Override Defaults: Yes
 Storm Duration(hrs): 3.00
 Rainfall File: Huff 3
 Rainfall Amount(in): 3.87

Time(hrs)	Print Inc(min)
1.000	15.00
3.000	5.00
5.000	15.00

 Name: 100-6hr
 Filename: D:\IN2005\0985-Shoe Carnival\ICPR\100-6hr.R32

Override Defaults: Yes
 Storm Duration(hrs): 6.00
 Rainfall File: Huff 3
 Rainfall Amount(in): 4.45

Time(hrs)	Print Inc(min)
2.000	15.00
6.000	5.00
9.000	15.00

 Name: 25-12hr
 Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-12hr.R32

Override Defaults: Yes
 Storm Duration(hrs): 12.00
 Rainfall File: Huff 3
 Rainfall Amount(in): 3.97

Time(hrs)	Print Inc(min)
3.000	15.00
8.000	5.00
15.000	15.00

 Name: 25-1hr
 Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-1hr.R32

Override Defaults: Yes
 Storm Duration(hrs): 1.00
 Rainfall File: Huff 3

Rainfall Amount (in): 2.08

Time (hrs)	Print Inc (min)
1.500	2.00
2.000	5.00

Name: 25-24hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-24hr.R32

Override Defaults: Yes
Storm Duration (hrs): 24.00
Rainfall File: Huff 3
Rainfall Amount (in): 4.66

Time (hrs)	Print Inc (min)
8.000	15.00
18.000	5.00
30.000	15.00

Name: 25-2hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-2hr.R32

Override Defaults: Yes
Storm Duration (hrs): 2.00
Rainfall File: Huff 3
Rainfall Amount (in): 2.80

Time (hrs)	Print Inc (min)
2.000	5.00
4.000	15.00

Name: 25-3hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-3hr.R32

Override Defaults: Yes
Storm Duration (hrs): 3.00
Rainfall File: Huff 3
Rainfall Amount (in): 3.06

Time (hrs)	Print Inc (min)
1.000	15.00
3.000	5.00
5.000	15.00

Name: 25-6hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-6hr.R32

Override Defaults: Yes
Storm Duration (hrs): 6.00
Rainfall File: Huff 3
Rainfall Amount (in): 3.53

Time (hrs)	Print Inc (min)
2.000	15.00
6.000	5.00
9.000	15.00

==== Routing Simulations =====

Name: 10-12hr Hydrology Sim: 10-12hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-12hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z (ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time (hrs): 0.000 End Time (hrs): 15.00
Min Calc Time (sec): 0.5000 Max Calc Time (sec): 60.0000
Boundary Stages: Boundary Flows:

Time (hrs)	Print Inc (min)
------------	-----------------

3.000 15.000
8.000 5.000
15.000 15.000

Group Run

BASE Yes

Name: 10-1hr Hydrology Sim: 10-1hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-1hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 2.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)

1.500 2.000
2.000 5.000

Group Run

BASE Yes

Name: 10-24hr Hydrology Sim: 10-24hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-24hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 30.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)

8.000 15.000
18.000 5.000
30.000 15.000

Group Run

BASE Yes

Name: 10-2hr Hydrology Sim: 10-2hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-2hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 4.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)

2.000 5.000
4.000 15.000

Group Run

BASE Yes

Name: 10-3hr Hydrology Sim: 10-3hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-3hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 5.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
1.000	15.000
3.000	5.000
5.000	15.000

Group Run

BASE Yes

Name: 10-6hr Hydrology Sim: 10-6hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\10-6hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 9.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
2.000	15.000
6.000	5.000
9.000	15.000

Group Run

BASE Yes

Name: 25-12hr Hydrology Sim: 25-12hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-12hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 15.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
3.000	15.000
8.000	5.000
15.000	15.000

Group Run

BASE Yes

Name: 25-1hr Hydrology Sim: 25-1hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-1hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000

Start Time (hrs): 0.000 End Time (hrs): 2.00
Min Calc Time (sec): 0.5000 Max Calc Time (sec): 60.0000
Boundary Stages: Boundary Flows:

Time (hrs) Print Inc (min)

1.500 2.000
2.000 5.000

Group Run

BASE Yes

Name: 25-24hr Hydrology Sim: 25-24hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-24hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z (ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time (hrs): 0.000 End Time (hrs): 30.00
Min Calc Time (sec): 0.5000 Max Calc Time (sec): 60.0000
Boundary Stages: Boundary Flows:

Time (hrs) Print Inc (min)

8.000 15.000
18.000 5.000
30.000 15.000

Group Run

BASE Yes

Name: 25-2hr Hydrology Sim: 25-2hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-2hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z (ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time (hrs): 0.000 End Time (hrs): 4.00
Min Calc Time (sec): 0.5000 Max Calc Time (sec): 60.0000
Boundary Stages: Boundary Flows:

Time (hrs) Print Inc (min)

2.000 5.000
4.000 15.000

Group Run

BASE Yes

Name: 25-3hr Hydrology Sim: 25-3hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-3hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z (ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time (hrs): 0.000 End Time (hrs): 5.00
Min Calc Time (sec): 0.5000 Max Calc Time (sec): 60.0000
Boundary Stages: Boundary Flows:

Time (hrs) Print Inc (min)

1.000 15.000
3.000 5.000

5.000 15.000

Group Run

BASE Yes

Name: 25 6hr Hydrology Sim: 25-6hr
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-6hr.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 9.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)

2.000 15.000
6.000 5.000
9.000 15.000

Group Run

BASE Yes

=====
=== Boundary Conditions ===
=====

**APPENDIX J:
EMERGENCY SPILLWAY CALCULATIONS**

EMERGENCY SPILLWAY, POND NORTH OF BUILDING
 SHOE CARNIVAL DISTRIBUTION CENTER
 JOB NUMBER IN2005-0985
 2/24/2006

WEIR FLOW CALCULATIONS

width = 20 feet
 Side Slope = 3 horiz. feet/ vert. feet
 Inv. elev. = 392.75 ft

20-foot wide spillway

Elevation, (ft)	h, (ft)	A, (ft ²)	Q, (cfs)
392.75	0.00	0.00	0.00
393.50	0.75	15.84	43.91
394.00	1.25	27.34	97.83
395.00	2.25	52.59	252.45
396.00	3.25	80.84	466.38

$$Q = CAh^{0.5}$$

$$C = 3.2$$

$$g = 32.2 \text{ ft/s}^2$$

EMERGENCY SPILLWAY, NORTHERNMOST POND
 SHOE CARNIVAL DISTRIBUTION CENTER
 JOB NUMBER IN2005-0985
 9/5/2006

WEIR FLOW CALCULATIONS

165-foot wide spillway

width = 165 feet
 Side Slope = 3 horiz. feet/ vert. feet
 Inv. elev. = 392.00 ft

Elevation, (ft)	h, (ft)	A (ft^2)	Q, (cfs)
392.00	0.00	0.00	0.00
392.50	0.50	82.88	187.52

$Q = CAh^{0.5}$

C = 3.2

g = 32.2 ft/s²

**APPENDIX K:
25-YEAR FLOOD ELEVATION CALCULATIONS**

Name	Group	Simulation	Max Time Stage hrs	Max Stage ft	Warning Stage ft	Max Delta Stage ft	Max Surf Area ft2	Max Time Inflow hrs	Max Inflow cfs	Max Time Outflow hrs	Max Outflow cfs
Existing Pond	BASE	25-12Off	8.91	391.46	389.00	0.0019	873340	8.50	127.69	8.93	114.68
Existing Pond	BASE	25-1cOff	1.99	391.24	389.00	0.0019	787848	1.27	190.68	1.98	47.02
Existing Pond	BASE	25-24Off	17.06	391.34	389.00	0.0019	824766	16.83	81.70	17.04	79.91
Existing Pond	BASE	25-2cOff	2.52	391.56	389.00	0.0019	910619	1.92	273.34	2.52	138.09
Existing Pond	BASE	25-3cOff	3.22	391.65	389.00	0.0019	946069	2.59	251.91	3.22	159.12
Existing Pond	BASE	25-6cOff	5.12	391.61	389.00	0.0019	930921	4.50	194.95	5.12	150.26

Estimated 25 year Flood Elevation

=====

==== Basins =====

Name: Offsite (Ex) Node: West of 57 Status: Onsite
Group: BASE Type: SCS Unit Hydrograph

Unit Hydrograph: Uh484 Peaking Factor: 484.0
Rainfall File: Storm Duration(hrs): 0.00
Rainfall Amount(in): 0.000 Time of Conc(min): 57.60
Area(ac): 252.000 Time Shift(hrs): 0.00
Curve Number: 83.00 Max Allowable Q(cfs): 999999.000
DCIA(%): 0.00

==== Nodes =====

Name: East 164 Base Flow(cfs): 0.000 Init Stage(ft): 391.160
Group: BASE Warn Stage(ft): 392.000
Type: Time/Stage

Time (hrs)	Stage (ft)
0.00	391.160
999999.00	391.160

Name: East of 57 Base Flow(cfs): 0.000 Init Stage(ft): 389.900
Group: BASE Warn Stage(ft): 398.940
Type: Stage/Area

Stage (ft)	Area (ac)

Name: Existing Pond Base Flow(cfs): 0.000 Init Stage(ft): 388.100
Group: BASE Warn Stage(ft): 389.000
Type: Stage/Area

Stage (ft)	Area (ac)
388.100	6.0600
389.000	6.1400
390.000	6.8500
391.000	15.8000

Name: West 164-a Base Flow(cfs): 0.000 Init Stage(ft): 387.000
Group: BASE Warn Stage(ft): 389.000
Type: Stage/Area

Stage (ft)	Area (ac)

Name: West 164-b Base Flow(cfs): 0.000 Init Stage(ft): 386.000
Group: BASE Warn Stage(ft): 389.000
Type: Stage/Area

Stage (ft)	Area (ac)

Name: West of 57 Base Flow(cfs): 0.000 Init Stage(ft): 390.650
Group: BASE Warn Stage(ft): 398.940
Type: Stage/Area

Stage (ft)	Area (ac)

```
=====
==== Cross Sections =====
=====
```

Name: SR 57
Encroachment: No
Group: BASE

Station(ft)	Elevation(ft)	Manning's N
0.000	399.220	0.035000
230.000	399.000	0.035000
265.000	398.940	0.035000
287.000	399.000	0.035000
310.000	400.000	0.030000

```
=====
==== Pipes =====
=====
```

Name: Pipe Out
Group: BASE
From Node: West 164-b
To Node: East 164
Length(ft): 301.00
Count: 1
Friction Equation: Average Conveyance
Solution Algorithm: Automatic
Flow: Both
Entrance Loss Coef: 0.50
Exit Loss Coef: 0.00
Bend Loss Coef: 0.00
Outlet Ctrl Spec: Use dc or tw
Inlet Ctrl Spec: Use dn
Stabilizer Option: None

UPSTREAM	DOWNSTREAM
Geometry: Circular	Circular
Span(in): 144.00	144.00
Rise(in): 144.00	144.00
Invert(ft): 380.010	379.160
Manning's N: 0.025000	0.025000
Top Clip(in): 0.000	0.000
Bot Clip(in): 0.000	0.000

Upstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall

Downstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall

```
-----
Name: SR 57 Culvert      From Node: West of 57      Length(ft): 52.00
Group: BASE              To Node: East of 57      Count: 1
```

Friction Equation: Average Conveyance
Solution Algorithm: Automatic
Flow: Both
Entrance Loss Coef: 0.50
Exit Loss Coef: 0.00
Bend Loss Coef: 0.00
Outlet Ctrl Spec: Use dc or tw
Inlet Ctrl Spec: Use dn
Stabilizer Option: None

UPSTREAM	DOWNSTREAM
Geometry: Rectangular	Rectangular
Span(in): 48.00	48.00
Rise(in): 60.00	60.00
Invert(ft): 390.650	389.900
Manning's N: 0.025000	0.025000
Top Clip(in): 0.000	0.000
Bot Clip(in): 6.000	6.000

Upstream FHWA Inlet Edge Description:
Rectangular Box: 45° wingwall flare d=.043D

Downstream FHWA Inlet Edge Description:
Rectangular Box: 45° wingwall flare d=.043D

```
=====
==== Channels =====
=====
```

Name: Ditch 57-Pond
Group: BASE
From Node: East of 57
To Node: Existing Pond
Length(ft): 90.00
Count: 1
Friction Equation: Average Conveyance
Solution Algorithm: Automatic
Flow: Both
Contraction Coef: 0.000
Expansion Coef: 0.000
Entrance Loss Coef: 0.500
Exit Loss Coef: 0.000
Outlet Ctrl Spec: Use dc or tw
Inlet Ctrl Spec: Use dn
Stabilizer Option: None

UPSTREAM	DOWNSTREAM
Geometry: Trapezoidal	Trapezoidal
Invert(ft): 389.900	388.100
TClpInitZ(ft): 9999.000	9999.000
Manning's N: 0.030000	0.030000
Top Clip(ft): 0.000	0.000
Bot Clip(ft): 0.000	0.000
Main XSec:	
AuxElev1(ft):	
Aux XSec1:	
AuxElev2(ft):	
Aux XSec2:	
Top Width(ft):	

Depth(ft):
 Bot Width(ft): 2.000 2.000
 LtSdSlp(h/v): 3.00 3.00
 RtSdSlp(h/v): 3.00 3.00

 Name: Ditch East-Out From Node: West 164-a Length(ft): 33.00
 Group: BASE To Node: West 164-b Count: 1

UPSTREAM	DOWNSTREAM	Friction Equation: Average Conveyance
Geometry: Trapezoidal	Trapezoidal	Solution Algorithm: Automatic
Invert(ft): 387.000	380.010	Flow: Both
TClpInitZ(ft): 9999.000	9999.000	Contraction Coef: 0.000
Manning's N: 0.030000	0.030000	Expansion Coef: 0.000
Top Clip(ft): 0.000	0.000	Entrance Loss Coef: 0.500
Bot Clip(ft): 0.000	0.000	Exit Loss Coef: 0.000
Main XSec:		Outlet Ctrl Spec: Use dc or tw
AuxElev1(ft):		Inlet Ctrl Spec: Use dn
Aux XSec1:		Stabilizer Option: None
AuxElev2(ft):		
Aux XSec2:		
Top Width(ft):		
Depth(ft):		
Bot Width(ft): 5.000	5.000	
LtSdSlp(h/v): 3.00	3.00	
RtSdSlp(h/v): 3.00	3.00	

 Name: Ditch Pond-East From Node: Existing Pond Length(ft): 275.00
 Group: BASE To Node: West 164-a Count: 1

UPSTREAM	DOWNSTREAM	Friction Equation: Average Conveyance
Geometry: Trapezoidal	Trapezoidal	Solution Algorithm: Automatic
Invert(ft): 388.100	387.000	Flow: Both
TClpInitZ(ft): 9999.000	9999.000	Contraction Coef: 0.000
Manning's N: 0.030000	0.030000	Expansion Coef: 0.000
Top Clip(ft): 0.000	0.000	Entrance Loss Coef: 0.500
Bot Clip(ft): 0.000	0.000	Exit Loss Coef: 0.000
Main XSec:		Outlet Ctrl Spec: Use dc or tw
AuxElev1(ft):		Inlet Ctrl Spec: Use dn
Aux XSec1:		Stabilizer Option: None
AuxElev2(ft):		
Aux XSec2:		
Top Width(ft):		
Depth(ft):		
Bot Width(ft): 5.000	5.000	
LtSdSlp(h/v): 3.00	3.00	
RtSdSlp(h/v): 3.00	3.00	

==== Weirs =====

Name: SR 57 Roadway From Node: West of 57
 Group: BASE To Node: East of 57
 Flow: Both Count: 1
 Type: Vertical: Paved Geometry: Irregular

XSec: SR 57
 Invert(ft): 398.940
 Control Elevation(ft): 398.940
 Struct Opening Dim(ft): 9999.00

TABLE

Bottom Clip(ft): 0.000
 Top Clip(ft): 0.000
 Weir Discharge Coef: 3.200
 Orifice Discharge Coef: 0.600

==== Hydrology Simulations =====

Name: 25-12off
 Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-12off.R32

Override Defaults: Yes
 Storm Duration(hrs): 12.00
 Rainfall File: Huff 3

Rainfall Amount(in): 3.97

Time(hrs)	Print	Inc(min)
3.000		15.00
8.000		5.00
15.000		15.00

Name: 25-1off
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-1off.R32

Override Defaults: Yes
Storm Duration(hrs): 1.00
Rainfall File: Huff 3
Rainfall Amount(in): 2.08

Time(hrs)	Print	Inc(min)
1.500		2.00
2.000		5.00

Name: 25-24off
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-24off.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Huff 3
Rainfall Amount(in): 4.66

Time(hrs)	Print	Inc(min)
8.000		15.00
18.000		5.00
30.000		15.00

Name: 25-2off
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-2off.R32

Override Defaults: Yes
Storm Duration(hrs): 2.00
Rainfall File: Huff 3
Rainfall Amount(in): 2.80

Time(hrs)	Print	Inc(min)
2.000		5.00
4.000		15.00

Name: 25-3off
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-3off.R32

Override Defaults: Yes
Storm Duration(hrs): 3.00
Rainfall File: Huff 3
Rainfall Amount(in): 3.06

Time(hrs)	Print	Inc(min)
1.000		15.00
3.000		5.00
5.000		15.00

Name: 25-6off
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-6off.R32

Override Defaults: Yes
Storm Duration(hrs): 6.00
Rainfall File: Huff 3
Rainfall Amount(in): 3.53

Time(hrs)	Print	Inc(min)
2.000		15.00
6.000		5.00
9.000		15.00

=====
=== Routing Simulations ===
=====

Name: 25-12off Hydrology Sim: 25-12off

Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-12off.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 15.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
3.000	15.000
8.000	5.000
15.000	15.000

Group	Run
BASE	Yes

Name: 25-1off Hydrology Sim: 25-1off
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-1off.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 2.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
1.500	2.000
2.000	5.000

Group	Run
BASE	Yes

Name: 25-24off Hydrology Sim: 25-24off
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-24off.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 30.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
8.000	15.000
18.000	5.000
30.000	15.000

Group	Run
BASE	Yes

Name: 25-2off Hydrology Sim: 25-2off
Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-2off.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 4.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000

Boundary Stages:

Boundary Flows:

Time (hrs)	Print Inc (min)
2.000	5.000
4.000	15.000

Group	Run
BASE	Yes

Name: 25-3off Hydrology Sim: 25-3off
 Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-3off.I32

Execute: Yes Restart: No Patch: No
 Alternative: No

Max Delta Z(ft): 1.00	Delta Z Factor: 0.00500
Time Step Optimizer: 10.000	
Start Time(hrs): 0.000	End Time(hrs): 5.00
Min Calc Time(sec): 0.5000	Max Calc Time(sec): 60.0000
Boundary Stages:	Boundary Flows:

Time (hrs)	Print Inc (min)
1.000	15.000
3.000	5.000
5.000	15.000

Group	Run
BASE	Yes

Name: 25-6off Hydrology Sim: 25-6off
 Filename: D:\IN2005\0985-Shoe Carnival\ICPR\25-6off.I32

Execute: Yes Restart: No Patch: No
 Alternative: No

Max Delta Z(ft): 1.00	Delta Z Factor: 0.00500
Time Step Optimizer: 10.000	
Start Time(hrs): 0.000	End Time(hrs): 9.00
Min Calc Time(sec): 0.5000	Max Calc Time(sec): 60.0000
Boundary Stages:	Boundary Flows:

Time (hrs)	Print Inc (min)
2.000	15.000
6.000	5.000
9.000	15.000

Group	Run
BASE	Yes

=====
 === Boundary Conditions ===
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