FINAL DRAINAGE REPORT for AmeriQual Group LLC Vanderburgh County, Indiana Project No.: 7296.1.003-B

October 05, 2022

Prepared For:

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Prepared By:

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10|05|2022



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Introduction

AmeriQual Group is proposing a new truck loading area at the southwest end of the existing property. The site is located at 18200 Hwy 41 N, Evansville, IN and is within the Highway 41 Impacted Drainage Area.. The site is within Section 29, Township 4 South, Range 10 West located in Scott Township, Vanderburgh County. Please refer to the location map provided within this report for further details.

Existing Conditions

Site Conditions

The site is an existing industrial site. The proposed area to be developed drains via sheet flow to the tributary of Pond Flat Ditch, south of the site. The current condition of the proposed site is gravel and lawn area with mild slopes.

Soils Information

The Soil Survey of Vanderburgh County indicates the soils to be Birds silt loam (Bd) with 0 to 2 percent slopes, frequently flooded. Please refer to the attached soils map.

Floodplain Information

The property lies within Zone AE. The site was scaled on the Flood Insurance Rate Map (FIRM) for Vanderburgh County, Indiana, Community Panel Number 18163C0045D, dated March 17, 2011. Please refer to the enclosed excerpt from this map.

Proposed Development

The proposed project is the addition of pavement for a truck loading area, parking and a detention basin. The expected proposed impervious surface included within the development is approximately 16,600 SF.

The proposed design of the drainage control system

The projected is within the Highway 41 Impacted Drainage Area, the basin was designed to hold a 100-year storm event.

The storm system is designed to closely replicate the existing drainage patterns while capturing a large portion of the runoff created from the site and discharging only a minimal amount of runoff undetained. Only a small portion along the east edge of the proposed improvements is expected to leave the site undetained to the south. Generally, these areas follow these drainage patterns in the existing condition.

The storm system is comprised of a dry detention basin with an outlet structure. In general, runoff from the proposed improvements will be directed to the basin via sheet flow. The basin will reduce the amount of post-developed runoff leaving the site to the allowable release rate, as determined by the associated pre-developed subbasins.



Results of the Analysis

Analysis of the pre-developed site delineated one (1) subbasin, as shown on the pre-developed subbasin exhibit. The Time of Concentration calculations and Q_{10} values are shown for the pre-developed subbasin. Detailed calculations are included as an attachment.

Pre-Developed Subbasin:

- Area = 0.83 acres
- C = 0.48
- T_c = 5.00 mins
- Q₁₀ = 2.65 CFS

For the analysis of post-developed runoff conditions, a 100-year storm event was analyzed. The storm water detention basin was sized for a 100-year storm event using the Form 800.

Once developed the project site will discharge most of its runoff into the dry basin. The detention basin will detain the stormwater runoff and slowly release it to the south towards the tributary of Pond Flat itch. A smaller area of the proposed development runoff will be undetained, the undetained runoff was accounted for in the calculation of the release rate from the detention basin.

- Developed undetained area = 0.1 acres
- Developed detained area = 0.73 acres
- Undetained runoff rate = 0.87 CFS
- Allowable Basin Release Rate = 1.78 CFS (2.65 CFS 0.87 CFS)
- Actual Basin Release Rate = 1.63 CFS

Basin Maintenance Report

This brief report will highlight the dry detention basins' design and maintenance in accordance with the latest Vanderburgh County Drainage Ordinance Section 13.04.440, Technical Memorandums and supplements. The dry basin slopes to the outlet structure and the water will discharge through the weir.

Maintenance of the basin shall include but is not limited to: mowing, removing debris and obstructions; removal of overgrown vegetation, mitigating erosion, and any other requirements set forth by the Vanderburgh County Drainage Board.

No tree limbs, trunks, refuse from legally burnt vegetation, nor construction waste, demolition materials, or other man-made material may be buried within the area in which an impounding structure will be located. Notice shall be placed on construction drawings noting the prohibition to the burying of any such materials.



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Maps showing individual drainage areas within the project subdivided for use in the analysis thereof

All calculations, and exhibits are enclosed within this report.

Summary

The proposed development is truck loading area, parking and a detention basin. This report has provided analysis and proposed conditions which ultimately lessen the overall drainage impact of the project site.

Total Pre-Developed Release $Q_{10} = 2.65$ CFS Basins Developed Release $Q_{100} = 1.63$ CFS Undetained Developed Release $Q_{100} = 0.87$ CFS Total Developed Release $Q_{100} = 2.50$ CFS

Overall, the developed project will release less stormwater runoff during the 100-year storm than its pre-developed area during the 10-year storm. The proposed improvements calculations meet and exceed the requirements for the Highway 41 Impacted Drainage Area.

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USDA Natural Resources

Conservation Service

Web Soil Survey National Cooperative Soil Survey

	MAP LEGEND	MAP INFORMATION
Area of Interest (AOI) Area of Intere	st (AOI) Stony Spot	The soil surveys that comprise your AOI were mapped at 1:15,800.
Soils Soil Map Unit	Polygons Very Stony Spot	Warning: Soil Map may not be valid at this scale. Enlargement of maps beyond the scale of mapping can ca
Soil Map Unit	Lines Other Points Special Line Features	misunderstanding of the detail of mapping and accuracy of line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more de
Special Point Features Blowout Blowout Borrow Pit	Water Features Streams and Canals	Please rely on the bar scale on each map sheet for map measurements.
Clay Spot	Transportation +++ Rails ssion Interstate Highways	Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
Gravel Pit Gravelly Spot	US Routes Major Roads	Maps from the Web Soil Survey are based on the Web Me projection, which preserves direction and shape but distort distance and area. A projection that preserves area, such
Landfill Lava Flow Marsh or swa	Local Roads Background Aerial Photography	Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified o
Mine or Quari	y Water	of the version date(s) listed below. Soil Survey Area: Vanderburgh County, Indiana Survey Area Data: Version 21, Sep 9, 2021
Perennial War	er	Soil map units are labeled (as space allows) for map scale 1:50,000 or larger.
Saline Spot		Date(s) aerial images were photographed: Jul 24, 2020–2020
 Severely Eroc Sinkhole 	ed Spot	The orthophoto or other base map on which the soil lines v compiled and digitized probably differs from the backgroun imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.
Slide or SlipSodic Spot		



Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Bd	Birds silt loam, 0 to 2 percent slopes, frequently flooded	0.8	100.0%
Totals for Area of Interest		0.8	100.0%



National Flood Hazard Layer FIRMette



Legend



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020



U.S. Fish and Wildlife Service National Wetlands Inventory

Wetlands Map



September 15, 2022

Wetlands



Estuarine and Marine Deepwater

Estuarine and Marine Wetland

- Freshwater Forested/Shrub Wetland
 - Freshwater Pond

Freshwater Emergent Wetland

Lake Other Riverine This map is for general reference only. The US Fish and Wildlife Service is not responsible for the accuracy or currentness of the base data shown on this map. All wetlands related data should be used in accordance with the layer metadata found on the Wetlands Mapper web site.







Peak Runoff Calculation

Pre-Developed Site

Area (Ac) =	0.83		Area (Sf) =	35,953							
Weighted Runoff Coefficient											
Surface	Area					с	A*c		7		
Structures & Pavement (<2%)	1,429	S.I	F. =	0.03	AC.	0.92	0.0	03	1		
Structures & Pavement (2-5%)	- Í	S.I	F. =	0.00	AC.	0.94	0.0	00	1		
Structures & Pavement (5-10%)		S.I	F. =	0.00	AC.	0.96	0.0	00	1		
Structures & Pavement (>10%)	5,010	S.I	=	0.12	AC.	0.98	0.1	11	1		
Gravel (25 yr Storm)		S.I	=	0.00	AC.	0.60	0.0	00	1		
Gravel (50-100 yr Storm)	11,736	S.I	=. =	0.27	AC.	0.65	0.1	18	1		
Lawn (<2%)	4,434	S.I	F. =	0.10	AC.	0.15	0.0	02	1		
Lawn (2-5%)	2,558	S.I	F. =	0.06	AC.	0.25	0.0	01			
Lawn (5-10%)		S.I	F. =	0.00	AC.	0.40	0.0	00	1		
Lawn (>10%)		S.I	F. =	0.00	AC.	0.55	0.0	00	1		
Woodland Flat (<2%)		S.I	₹. =	0.00	AC.	0.12	0.0	00			
Woodland Flat (2-5%)		S.I	₹. =	0.00	AC.	0.24	0.0	00			
Woodland Rolling (5-10%)		S.I	₹. =	0.00	AC.	0.36	0.0	00			
Woodland Hilly (10-30%)		S.I	=	0.00	AC.	0.48	0.0	00			
Pasture Flat (<2%)		S.I	₹. =	0.00	AC.	0.12	0.0	00			
Pasture Flat (2-5%)		S.I	=. =	0.00	AC.	0.24	0.0	00			
Pasture Rolling (5-10%)		S.I	=. =	0.00	AC.	0.36	0.0	00			
Pasture Hilly (>10%)		S.I	=. =	0.00	AC.	0.48	0.0	00			
Cultivated (<2%)	10,786	S.I	=. =	0.25	AC.	0.20	0.0	05			
Cultivated (2-5%)		S.I	=. =	0.00	AC.	0.35	0.0	00			
Cultivated (5-10%)		S.I	=. =	0.00	AC.	0.50	0.0	00			
Cultivated (>10%)		S.I	=. =	0.00	AC.	0.65	0.0	00			
Bare Soil		S.I	=. =	0.00	AC.	0.72	0.0	00			
Water		S.I	=. =	0.00	AC.	1.00	0.0	00			
				0.83			0.4	40			
Wc =	0.48										
Time of Concentration											
Overland Flow											
Longth L (max 200ff)		_	01	foot			t	_	Overland Flow T	-	
		-	91	leet			۰ ۲	-		U.5*	(C ^{0.4})1
Slope, S		=	1.70%				ι _ο	=	[0.42 (L) (II)	J/[F)	(S)
Manning Coefficient, n	Lawn	=	0.035				to	=	2.98 min		
P _{2/24}		=	3.3								
o											
Shallow Flow				. .							
Length, L (Paved or Unpaved)	Unpaved	=	0	feet			V	=	16.1345*(S0.5)		
Slope, S		=	1.00%					=	1.613 ft/s =	96.81	ft/m
Velocity, V		=	1.61	ft/sec			t _s	=	Shallow Flow To	;	
							ts	=	(L/V) = 0.00	min	
Shallow Flow									. ,		
Length, L (Paved or Unpaved)	Unpaved	=	0	feet			V	=	16.1345*(S0.5)		
Slope, S	-	=	1.00%					=	1.613 ft/s =	96.81	ft/m
Velocity V		=	1 61	ft/sec			t.	=	Shallow Flow To	:	
volocity, v			1.01	10000			+	_		min	
							۲s	-	(L/V) = 0.00		
		t =	Total Time	of Concent	ation						
		t =	Σ to + Σ ts +	Σtc	auon						
			2.0 2.0	/Mip E Mi	nutoc)						
			0.05	Hour	nutesj						
Intensity (Vanderburgh Co.)											
		l ₂ =	5.02	in/hr							
		L. =	6.66	in/hr							
		•10 I	7.91	in/hr							
		25 -	0.05	in/hr							
Pook Pupoff Poto		100 -	9.95	m/nr							
rean NUIIVII Nale	O = CiA										
	Q _{yr} – CIA	_									
		$Q_2 =$	<u>2.00</u>	cts							
		ຊ ₁₀ =	<u>2.65</u>	cfs							
		Q ₂₅ =	<u>3.10</u>	cfs							
	C	l ₁₀₀ =	<u>3.96</u>	cfs							

1345*(S0.5) 13 ft/s = 96.81 ft/min llow Flow Tc)= 0.00 min

1345*(S0.5) 13 ft/s = 96.81 ft/min llow Flow Tc) = 0.00 min





POST-DEVELOPED WEIGHTED RUNOFF COEFFICIENTS

Project:AmeriqualProject #7296Engineer:JAE

County: Vanderburgh Date: 9/28/22

С Surface Undetained 5 6 7 9 1 3 4 8 10 Structures & Pavement (<2%) 0.92 12,813 4,150 Structures & Pavement (2-5%) 0.94 Structures & Pavement (5-10%) 0.96 Structures & Pavement (>10%) 0.98 5,010 Gravel (10 yr storm) 0.50 Gravel (25 yr storm) 0.60 Gravel (50-100 yr storm) 0.65 Lawn (<2%) 0.15 10,286 Lawn (2-5%) 0.25 Lawn (5-10%) 0.40 Lawn (>10%) 0.55 Woodland Flat (<2%) 0.12 Woodland Flat (2-5%) 0.24 Woodland Rolling (5-10%) 0.36 Woodland Hilly (>10%) 0.48 Pasture Flat (<2%) 0.12 Pasture Flat (2-5%) 0.25 Pasture Rolling (5-10%) 0.36 Pasture Hilly (>10%) 0.48 Cultivated Flat (<2%) 0.20 Cultivated Flat (2-5%) 0.35 Cultivated Rolling (5-10%) 0.50 Cultivated Hilly (>10%) 0.65 Bare Soil 0.72 Water 1.00 3,744

SUB-BASINS (S.F.)

Total SF	31,853	4,150	-	-	-	-	-	-	-	-
Total Acres	0.73	0.10								
Weighted C	0.69	0.92								

SUB-BASIN CALCULATIONS

Indiana LTAP Stormwater Drainage Manual

Project: Ameriqual Project # JAE Engineer:

7296

County: Vanderburgh Date: 9/28/22

Design Period: 100 year

Sub-basin	Area (ac.)	Weighted C	Overland Flow L (ft)	Overland Flow ∆H (ft)	Overland Flow S (%)	Overland Flow t _o (mins)	Shallow Flow L (ft)	Shallow Flow	Shallow Flow Type (Paved/Unpaved)	Shallow Flow S (%)	Shallow Flow t _s (mins)	Total Time of Conc. t _c (mins)	l(10) (in/hr)	Q(10) (CFS)	l(100) (in/hr)	Q(100) (CFS)
1	0.73	0.69	91	1.6	1.70%	5.89	21	0.4	Р	1.90%	0.12	6.02	6.36	3.21	9.51	4.80
Undetained	0.10	0.92	30	0.3	1.00%	1.77	0	0.0		0.00%	0.00	5.00	6.66	0.58	9.95	0.87

VANDERBURGH COUNTY DRAINAGE BOARD FORM 800										
PROJECT:	Ameriqual		DETENTI	ON FACILITY	/ DESIGN RET	URN PERIOD:	100	YRS		
DESIGNER:	JAE	7296		URN PERIOD:	10	YRS				
UNDEVELO	PED WATER		A (Au)			0.83		ACRES		
TIME OF CO			VELOPED W	ATERSHED)	5.00		MINUTES		
		1): E OOEEEIO				6.655		INCHES/HR		
			$= O(x^*) \cdot (Cu):$			0.48	0.65	050		
			= CuriurA): Nd) — contribu	ting to booin		0.72	2.05			
			uannoo - contribu ד (כא)י	ling to basin		0.73		ACRES		
			n (Ca).			0.09	0.07	059		
							0.07			
			200				1.70	CF3 CFS		
ACTUAL DI			JVV				1.05	65		
STORM	RAINFALL	INFLOW	OUTFLOW	STORAGE	REQUIRED					
DURATION	INTENSITY	RATE	RATE	RATE	STORAGE					
Td	ld	I(Td)	0							
14	14	(Cd*ld*Ad)	(actual)	Q-(bT)I	(I(Td)-Q)*Td/12					
(HRS)	(INCH/HR)	(CFS)	(CFS)	(CFS)	(ACRE.FT)					
0.08	9.951	5.01	1.63	3.38	0.02					
0.17	8.053	4.06	1.63	2.43	0.03					
0.25	6.677	3.36	1.63	1.73	0.04					
0.50	4.214	2.12	1.63	0.49	0.02					
0.67	3.289	1.66	1.63	0.03	0.00					
0.75	2.943	1.48	1.63	-0.15	-0.01					
1.00	2.936	1.48	1.63	-0.15	-0.01					
1.50	2.252	1.13	1.63	-0.50	-0.06					
2.00	1.844	0.93	1.63	-0.70	-0.12					
2.50	1.571	0.79	1.63	-0.84	-0.17					
3.00	1.374	0.69	1.63	-0.94	-0.23					
4.00	1.107	0.56	1.63	-1.07	-0.35					
5.00	0.933	0.47	1.63	-1.16	-0.48					
6.00	0.811	0.41	1.63	-1.22	-0.61					
7.00	0.719	0.36	1.63	-1.27	-0.73					
8.00	0.648	0.33	1.63	-1.30	-0.86					
9.00	0.590	0.30	1.63	-1.33	-0.99					
10.00	0.543	0.27	1.63	-1.36	-1.12					
	•					I				
		PEAK STO	RAGE (ACRI	E-FT)	0.04					
		PEAK STO	RAGE (CUBI	C FT)	1,560					

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BASIN DISCHARGE AND STORAGE VOLUME									
Orifice:	$Q = C_d A_0 \sqrt{2}$	$\sqrt{2gh_0}$ LTAP 6.3.2							
Pipe Dia. D (inch)	8 6.00	(inch) Orifice diameter)						
$A_{s}(s.f.)$	0.20	Area of orifice	1	D	→ Q				
$a_{\rm o}(f/s^2)$	32.2	Acceleration due to ar	avity	T T	Free Jet				
9 (1/3) H (ft)	2 00	Head at Inlet		1					
h_{α} (ft)	1.75	Head at center of orific	e /						
C,	0.61	Discharge coefficient							
- u	0101	Disentarge seemolent							
Q=	1.27	CFS		X	Not used				
Pipe Flow:	1	$h \qquad 1/2$							
	$Q = A_P \left(\underline{\underline{K}} \right)$	$\frac{\frac{n_p}{e + K_0}}{2g} + \frac{2.87n^2L}{D^{4/3}} \right)$	LTAP 6.3.5						
Pipe Dia.	8	(inch)							
Ap (s.f.)	0.35	Area of Pipe							
n	0.012	Manning roughness co	oef.						
g (f/s²)	32.2	Acceleration due to gra	avity						
H (ft)	2.00	Head at invert							
h _¤ (ft)	1.67	Head at center of pipe							
L (ft)	13	Length of pipe							
K _e	0.85	Entrance Loss							
K _o	1.00	Outlet Loss							
0-	2 31	CES							
Q-	2.01	010		X	Not used				
Rectangula	r Weir:) 2/		-					
	($Q = \frac{2}{3}C_d\sqrt{2g}Lh_w^{3/2}$	LTAP 6.3.3						
L (ft)	0.5	Length of the weir			- 50				
g (t/s²)	32.2	Acceleration due to gra	avity						
n _w (it)	1.00	Head above welf							
Ud	0.61	Discharge coefficient							
Q=	1.63	CFS			Not used				
Storage:				I					
	Stage	Surface Area (S.F.)	Cum. Storage Vol. (C.F.)	Notes					
	432	0		Outlet					
	433	3,244	1,622	100 Year Water Elev					
	433.2	7,630	2,709	TOB Elevation	_i				
		Available Storage:	2,709	0.06 AC-FT	4				
		Required: (Peack x 10%)	1,716	0.04 AC-FT	-				
			63%	Basin Capacity]				
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